

AS2070: Aerospace Structural Mechanics Module 2: Composite Material Mechanics

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Chapters 1-3in Gibson (2012).

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PRINCIPLES OF

COMPOSITE Material

MECHANICS

Chapters 1-3, 11 in Kollár and Springer (2003).

MECHANICS OF



Chapter 25 in Megson (2013)

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(Also see Daniel and Ishai 2006)

Introduction

- Structural material consisting of multiple non-soluble macro-constituents.
- Main motivation: material properties tailored to applications.
- Both stiffness and strength comes from the fibers/particles, and the matrix holdes everything together.



Types of composite materials (Figure from NPTEL Online-IIT KANPUR (2025))

Examples

- Reinforced concrete
- Wood (lignin matrix reinforced by cellulose fibers)
- Carbon-Fiber Reinforced Plastics (CFRP)

Introduction

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Continuous fibres

Short fibres/Whiskers

Particulate

Types of composite materials (Figure from NPTEL Online-IIT KANPUR (2025))

Examples

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Introduction

• Structural material consisting of multiple non-soluble macro-constituents.



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Introduction



1.2. Modeling Composite Material

Introduction

Two main approaches:



1.2. Modeling Composite Material

Introduction

Two main approaches:



Introduction



• Strain is fixed, but stress experienced by media differ.

 $\sigma_l = E_l \varepsilon_l$

• Stress-strain relationship simplifies as,

$$\sigma_m = E_m \varepsilon_l, \quad \sigma_f = E_f \varepsilon_l$$
$$\sigma_l A = \sigma_m A_m + \sigma_f A_f$$
$$\implies \boxed{E_l = \frac{A_f}{A} E_f + \frac{A_m}{A} E_m}.$$

Introduction



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$$\begin{split} \sigma_m &= E_m \varepsilon_l, \quad \sigma_f = E_f \varepsilon_l \\ \sigma_l A &= \sigma_m A_m + \sigma_f A_f \\ \implies \boxed{E_l &= \frac{A_f}{A} E_f + \frac{A_m}{A} E_m}. \end{split}$$

Transverse Elongation



• Stress is fixed, strains differ:

$$\begin{split} \varepsilon_t l_t &= \varepsilon_m l_m + \varepsilon_f l_f \\ \Longrightarrow \; \frac{\sigma_t}{E_t} l_t &= \frac{\sigma_t}{E_m} l_m + \frac{\sigma_t}{E_f} l_f \\ \Longrightarrow & \boxed{\frac{1}{E_t} = \frac{1}{E_m} \frac{l_m}{l_t} + \frac{1}{E_f} \frac{l_f}{l_t}}. \end{split}$$

(Figures	from	Megson	2013)
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Introduction: Poisson Effects



(Figures from Megson 2013)

Introduction: Poisson Effects



(Figures from Megson 2013)

Introduction: Poisson Effects



Introduction: Anisotropy

General Anisotropy

σ_{xx}		C_{11}	C_{12}	C_{13}	C_{14}	C_{15}	C_{16}	ε_{xx}
σ_{yy}		C_{12}	C_{22}	C_{23}	C_{24}	C_{25}	C_{26}	ε_{yy}
σ_{zz}	_	C_{13}	C_{23}	C_{33}	C_{34}	C_{35}	C_{36}	ε_{zz}
σ_{xy}	_	C_{14}	C_{24}	C_{34}	C_{44}	C_{45}	C_{46}	γ_{xy}
σ_{xz}		C_{15}	C_{25}	C_{35}	C_{45}	C_{55}	C_{56}	γ_{xz}
σ_{yz}		C_{16}	C_{26}	C_{36}	C_{46}	C_{56}	C_{66}	γ_{yz}

Introduction: Anisotropy

General Anisotropy

$$\begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{xy} \\ \sigma_{xz} \\ \sigma_{yz} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & C_{14} & C_{15} & C_{16} \\ C_{12} & C_{22} & C_{23} & C_{24} & C_{25} & C_{26} \\ C_{13} & C_{23} & C_{33} & C_{34} & C_{35} & C_{36} \\ C_{14} & C_{24} & C_{34} & C_{44} & C_{45} & C_{46} \\ C_{15} & C_{25} & C_{35} & C_{45} & C_{55} & C_{56} \\ C_{16} & C_{26} & C_{36} & C_{46} & C_{56} & C_{66} \end{bmatrix} \begin{bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \varepsilon_{zz} \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{bmatrix}$$

Monoclinic: Single Plane of Symmetry

$$\begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{xy} \\ \sigma_{xz} \\ \sigma_{yz} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & C_{14} & 0 & 0 \\ C_{12} & C_{22} & C_{23} & C_{24} & 0 & 0 \\ C_{13} & C_{23} & C_{33} & C_{34} & 0 & 0 \\ C_{14} & C_{24} & C_{34} & C_{44} & 0 & 0 \\ 0 & 0 & 0 & 0 & C_{55} & C_{56} \\ 0 & 0 & 0 & 0 & C_{56} & C_{66} \end{bmatrix} \begin{bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \varepsilon_{zz} \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{bmatrix}$$

Introduction: Anisotropy



Transversely Isotropic

$$\begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{xy} \\ \sigma_{xz} \\ \sigma_{yz} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & 0 & 0 & 0 \\ C_{12} & C_{22} & C_{13} & 0 & 0 & 0 \\ C_{13} & C_{13} & C_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & C_{44} & 0 & 0 \\ 0 & 0 & 0 & 0 & C_{44} & 0 \\ 0 & 0 & 0 & 0 & 0 & \frac{C_{11} - C_{12}}{2} \end{bmatrix} \begin{bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \varepsilon_{zz} \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{bmatrix}$$

1.4. Classical Laminate Theory

Introduction



2. Composite Materials



Griffith's experiments with glass fibres (1920) (Figure from Gibson 2012)

2. Composite Materials



2.1. Types of Composite Materials

Composite Materials



FIGURE 1.4

Types of fiber-reinforced composites. (a) Continuous fiber composite, (b) woven composite, (c) chopped fiber composite, and (d) hybrid composite.

(Figure from Gibson 2012)

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Micro-Mechanics Descriptions

The rule of mixtures is introduced as a very simple framework for developing "overall"/representative mechanical properties.

Basic Definitions

Subscripts $(\cdot)_f$, $(\cdot)_m$, $(\cdot)_v$, and $(\cdot)_c$ denote quantities corresponding to the fiber, matrix, void, and composite (as a whole).

Volume Fraction $v_f = \frac{V_f}{V_c}, v_m = \frac{V_m}{V_c}, v_v = \frac{V_v}{V_c}$ such that $v_f + v_m + v_v = 1$. Note that composite density $\rho_c = \rho_f v_f + \rho_m v_m$.

Weight Fraction $w_f = \frac{\rho_f}{\rho_c} v_f$



(Figure 3.5a from Gibson 2012)

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 $E_1 = v_f E_f + v_m E_m$ $(\times)E_2 = \left(\frac{v_f}{E_f} + \frac{v_m}{E_m}\right)^{-1}$ $\nu_{12} = v_f \nu_f + v_m \nu_m$ $(\times)G_{12} = \left(\frac{v_f}{G_f} + \frac{v_m}{G_m}\right)^{-1}$

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Micro-Mechanics Descriptions



Micro-Mechanics Descriptions

• The mismatch is related to the fact that our idealized picture was a poor representation of reality to begin with. More geometrical details of the fiber arrangement are necessary.



Micro-Mechanics Descriptions

(Recommended reading: Sec. 3.2.3 in Daniel and Ishai 2006)

The Halpin-Tsai Equation

$$E_{2} = E_{m} \frac{1 + \xi \eta v_{f}}{1 - \eta v_{f}}, \quad \eta = \frac{E_{f} - E_{m}}{E_{f} + \xi E_{m}}$$
$$= E_{m} \frac{E_{f} + \xi E_{m} + \xi v_{f} (E_{f} - E_{m})}{E_{f} + \xi E_{m} - v_{f} (E_{f} - E_{m})}$$

Note: $\xi = 2$ for circular section fibers. $\xi = \frac{2a}{b}$ for rectangular fibers (*b* being loaded side).

Case 1:
$$\xi \to 0$$

 $E_2 = \left(\frac{v_f}{E_f} + \frac{1 - v_f}{E_m}\right)^{-1}$
Series, *Reuss* model.
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Micro-Mechanics Descriptions



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3.2. Numerical Example

Micro-Mechanics Descriptions

(from Kollár and Springer 2003)

Consider a Graphite/Epoxy unidirectional ply. Matrix properties are given with subscript m in the table below. Nominal properties with fiber volume fraction $v_f = 60\%$ are also given. Assume that the fibers show anisotropy $(E_{f1} \neq E_{f2}).$

	E_1	E_2	G_{12}	ν_{12}	E_m	G_m	ν_m
Value	148	9.65	4.55	0.3	4.1	1.5	0.35

All modulii in GPa.

Estimate the following:

- Fiber modulus properties
- Composite material modulii for volume fraction $v_f = 0.55$.

(Also discussed sensitivity analysis)

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Material Symmetry and Anisotropy

Material Symmetry

The study of material symmetry is concerned with finding answers to the question: If the strain field on a deformable object is changed, how does the stress field change?

Material Symmetry and Anisotropy

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Consider the following Deformation Fields



Deformation Case 1

Deformation Case 2 (Case 1 Rotated)

Material Symmetry and Anisotropy

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Consider the following Deformation Fields

Material Symmetry and Anisotropy

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Material Symmetry and Anisotropy

Material Symmetry

The study of material symmetry is concerned with finding answers to the question: If the strain field on a deformable object is changed, how does the stress field change?



Macro-Mechanics Descriptions

How do stresses and strains transform under coordinate change?

- Suppose $\underline{x} \in \mathbb{R}^3$ are the coordinates of a point in 3D space.
- Let $\underline{x}' \in \mathbb{R}^3$ be the coordinates under transformation.
- We will write: $\underline{x}' = \underline{Q} \underline{x}$, with $\underline{Q}^{-1} = \underline{Q}^T$.

Strains

•
$$\underline{\underline{\varepsilon}} = \frac{1}{2} \left(\underline{\nabla}_{\underline{x}} \underline{u} + \underline{\nabla}_{\underline{x}} \underline{u}^T \right)$$

• $\underline{\nabla}_{\underline{x}'} \underline{u}' = \underline{Q} \, \underline{\nabla}_{\underline{x}} \underline{u} \underline{Q}^{-1}$
 $\implies \underline{\underline{\varepsilon}}' = \underline{Q} \, \underline{\varepsilon} \, \underline{Q}^T$.

Stresses

• Cauchy Stress Definition: $\underline{t} = \underline{\sigma} \underline{n}$

•
$$\underline{Q} \underline{t} = \underline{t}' = \underline{\underline{\sigma}}' \underline{\underline{n}}' = \underline{\underline{\sigma}}' \underline{Q} \underline{\underline{n}} = \underline{Q} \underline{\underline{\sigma}} \underline{\underline{n}}$$

 $\implies \underline{\underline{\sigma}}' = \underline{Q} \underline{\underline{\sigma}} \underline{Q}^T$

Reflections

Note that reflections may be expressed as a coordinate change with $\underline{\underline{Q}} = \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & -1 \end{bmatrix}$ (reflection about the *xy* plane).

Macro-Mechanics Descriptions

 $\bullet\,$ Under reflection about the xy plane, the strain transforms as,

$$\begin{bmatrix} \varepsilon'_x & \frac{\gamma'_{xy}}{2} & \frac{\gamma'_{xz}}{2} \\ \varepsilon'_y & \frac{\gamma'_{yz}}{2} \\ \text{sym} & \varepsilon'_z \end{bmatrix} = \begin{bmatrix} 1 & \cdot & \cdot \\ \cdot & 1 & \cdot \\ \cdot & \cdot & -1 \end{bmatrix} \begin{bmatrix} \varepsilon_x & \frac{\gamma_{xy}}{2} & \frac{\gamma_{xz}}{2} \\ \varepsilon_y & \frac{\gamma_{yz}}{2} \\ \text{sym} & \varepsilon_z \end{bmatrix} \begin{bmatrix} 1 & \cdot & \cdot \\ \cdot & 1 & \cdot \\ \cdot & \cdot & -1 \end{bmatrix}$$
$$= \begin{bmatrix} \varepsilon_x & \frac{\gamma_{xy}}{2} & -\frac{\gamma_{xz}}{2} \\ \varepsilon_y & -\frac{\gamma_{yz}}{2} \\ \text{sym} & \varepsilon_z \end{bmatrix}$$



Macro-Mechanics Descriptions

 $\bullet\,$ Under reflection about the xy plane, the strain transforms as,

If a material were symmetric about the xy plane, then reflecting the strain field about the xy plane will result in a stress field that is reflected about the same xy plane.

Note

- Strain field reflection is a <u>kinematic operation</u>/configuration change.
- Change in the Stress field is the <u>effect that the above</u> kinematic change results in.
- If the material happens to be symmetric about the reflection plane, then this change will be a reflection.

 $[\tau'_{yz}]$

Similarly for Stress

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 σ_x

 σ_y

 σ_z

 $\begin{bmatrix} Txy \\ Txz \\ Tyz \end{bmatrix}$

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 ε'_x ε'_y ε'_y

 $\gamma'_{xy} \\ \gamma'_{xz}$

Macro-Mechanics Descriptions

• We have said the following :



Recall that this symmetry follows from strain energy existence

$$\begin{bmatrix} \sigma_{j}^{\prime} \\ \sigma_{y}^{\prime} \\ \sigma_{z}^{\prime} \\ \tau_{xy}^{\prime} \\ \tau_{yz}^{\prime} \\ \tau_{yz}^{\prime} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & C_{14} & C_{15} & C_{16} \\ & C_{22} & C_{23} & C_{24} & C_{25} & C_{26} \\ & & C_{33} & C_{34} & C_{35} & C_{36} \\ & & & C_{44} & C_{45} & C_{46} \\ & & & & & C_{55} & C_{56} \\ & & & & & & C_{66} \end{bmatrix} \begin{bmatrix} \varepsilon_{j}^{\prime} \\ \varepsilon_{j}^{\prime}$$

(The $\underline{\underline{C}}$ matrix is the same in both the original and the reflected coordinate systems)

Macro-Mechanics Descriptions

We have	bies or	the fc	llowing	r•						
This leads to										
$\begin{bmatrix} C_{11} & C_1 \\ & C_2 \\ & & \\ & & \\ & & \text{syn} \end{bmatrix}$	$\begin{array}{cccc} & C_{13} & \\ & C_{23} & \\ & C_{33} & \\ & & \end{array}$ n	$C_{14} \\ C_{24} \\ C_{34} \\ C_{44}$	$-C_{15} - C_{25} - C_{35} - C_{45} - C_{45} - C_{55}$	$ \begin{bmatrix} -C_{16} \\ -C_{26} \\ -C_{36} \\ -C_{46} \\ C_{56} \\ C_{66} \end{bmatrix} = $	$=\begin{bmatrix} C_{11} \\ \\ \\ \\ \\ \end{bmatrix}$	C_{12} C_{22} sym	$C_{13} \\ C_{23} \\ C_{33}$	$C_{14} \\ C_{24} \\ C_{34} \\ C_{44}$	$C_{15} \\ C_{25} \\ C_{35} \\ C_{45} \\ C_{55}$	$\begin{array}{c} C_{16} \\ C_{26} \\ C_{36} \\ C_{46} \\ C_{56} \\ C_{66} \end{array}$

Recall that this symmetry follows from strain energy existence



 $(The \underline{\underline{C}} matrix is the same in both the original and the reflected coordinate systems)$

Macro-Mechanics Descriptions



Recall that this symmetry follows from strain energy existence



 $(The \underline{\underline{C}} matrix is the same in both the original and the reflected coordinate systems)$

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Macro-Mechanics Descriptions



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Macro-Mechanics Descriptions





Macro-Mechanics Descriptions



4.1. Material Symmetry and Anisotropy: Transverse Isotropy

Macro-Mechanics Descriptions

• In continuous fiber reinforced composites, it is often the case that the fibers are randomly distributed on a plane. This leads to planar isotropy in the plane perpendicular to the fiber stacking direction.



⁽Figure 2.6 from Gibson 2012)

• How do the stresses and strains transform on the plane?



$$\begin{split} & (\sigma_x, \sigma_y, \sigma_z, \tau_{xy}, \tau_{xz}, \tau_{yz}) \to (\sigma_\xi, \sigma_\eta, \sigma_z, \tau_{\xi\eta}, \tau_{\xiz}, \tau_{\etaz}) \\ & (\varepsilon_x, \varepsilon_y, \varepsilon_z, \gamma_{xy}, \gamma_{xz}, \gamma_{yz}) \to (\varepsilon_\xi, \varepsilon_\eta, \varepsilon_z, \gamma_{\xi\eta}, \gamma_{\xiz}, \gamma_{\etaz}) \end{split}$$

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Macro-Mechanics Descriptions

• The stresses and strains transform as follows on the plane:

- For an orthotropic material, the straight stresses/strains and shear stresses/strains are fully decoupled.
- So we will consider different cases of kinematic deformation fields to see if more can be said.







Macro-Mechanics Descriptions

4. Pure In-Plane Shear
$$(\gamma_{xy} \neq 0)$$

• From the constitutive properties we have $\tau_{xy} = C_{44}\gamma_{xy}$.
• Using this all the other components can be written as
 $\sigma_{\xi} = \sigma_{\eta} = \sigma_{\xi} = C_{44}\gamma_{xy} \sin 2\theta = C_{11}\varepsilon_{\xi} + C_{12}\varepsilon_{\eta}$ $\varepsilon_{\xi} = \frac{\gamma_{xy}}{2} \sin 2\theta$
 $\sigma_{\eta} = -C_{44}\gamma_{xy} \sin 2\theta = C_{12}\varepsilon_{\xi} + C_{11}\varepsilon_{\eta}$ $\varepsilon_{\eta} = -\frac{\gamma_{xy}}{2} \sin 2\theta$
 $\sigma_{z} = 0$ $\varepsilon_{z} = 0$
 $\tau_{\xi\eta} = C_{44}\gamma_{xy} \cos 2\theta$ $\varepsilon_{z} = 0$
 $\tau_{\xiz} = \tau_{\eta z} = 0$.
• So we have $C_{44}\gamma_{xy} \sin 2\theta = \frac{C_{11}-C_{12}}{2}\gamma_{xy} \sin 2\theta$. Therefore,
 $\left[\begin{array}{c} C_{11} & C_{12} & C_{13} & 0 & 0 & 0\\ C_{13} & 0 & 0 & 0\\ C_{33} & 0 & 0 & 0\\ C_{33} & 0 & 0 & 0\\ C_{55} & 0\\ \end{array} \right]$ to see if
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Macro-Mechanics Descriptions



4.1. Material Symmetry and Anisotropy: Engineering Constants

Macro-Mechanics Descriptions

- In engineering practice, the constants are usually written easier in terms of compliance.
- For a specially orthotropic material the strain-stress relationship are usually expressed as,

$$\begin{bmatrix} \varepsilon_x \\ \varepsilon_y \\ \varepsilon_z \\ \gamma_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{bmatrix} = \begin{bmatrix} \frac{1}{E_{11}} & -\frac{\nu_{21}}{E_{22}} & -\frac{\nu_{31}}{E_{33}} & 0 & 0 & 0 \\ -\frac{\nu_{12}}{E_{11}} & \frac{1}{E_{22}} & -\frac{\nu_{32}}{E_{33}} & 0 & 0 & 0 \\ -\frac{\nu_{13}}{E_{11}} & -\frac{\nu_{23}}{E_{22}} & \frac{1}{E_{33}} & 0 & 0 & 0 \\ & & & \frac{1}{G_{12}} & 0 & 0 \\ & & & & \frac{1}{G_{13}} & 0 \\ & & & & & \frac{1}{G_{23}} \end{bmatrix} \begin{bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{xz} \\ \tau_{yz} \end{bmatrix}$$

5. Analysis of Planar Laminates

• Let us just consider one thin layer of a transversely isotropic material (continuously reinforced composite along a single direction).



5. Analysis of Planar Laminates

• Let us just consider one thin layer of a transversely isotropic material (continuously reinforced composite along a single direction).



We invoke plane stress assumptions, setting σ_z = 0. Let us also assume small shears, τ_{xz} = 0, τ_{yz} = 0.
(Note: ε_z is not zero, and is implicitly defined)

$$\begin{bmatrix} \sigma_1 \\ \sigma_2 \\ \tau_{12} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & 0 \\ C_{12} & C_{22} & 0 \\ 0 & 0 & C_{33} \end{bmatrix} \begin{bmatrix} \varepsilon_1 \\ \varepsilon_2 \\ \gamma_{12} \end{bmatrix} (4 \text{ constants})$$
(Note change in notation in C_{ij})

Analysis of Planar Laminates



Positive $\boldsymbol{\theta}$

(Figure 2.11 from Gibson 2012)

$$\begin{split} & \sigma_x \\ \sigma_y \\ \tau_x y \end{bmatrix} = \underbrace{ \begin{bmatrix} \cos^2 \theta & \sin^2 \theta & -2\cos\theta \sin\theta \\ \sin^2 \theta & \cos^2 \theta & 2\cos\theta \sin\theta \\ \cos\theta \sin\theta & -\cos\theta \sin\theta & \cos^2 \theta - \sin^2 \theta \end{bmatrix}}_{\underline{T} = \begin{bmatrix} \cos^2 \theta & \sin^2 \theta & 2\cos\theta \sin\theta \\ \sin^2 \theta & \cos^2 \theta & -2\cos\theta \sin\theta \\ -\cos\theta \sin\theta & \cos\theta \sin\theta & \cos^2 \theta - \sin^2 \theta \end{bmatrix} \end{split}$$

- What if the coordinate system is not aligned with the fiber axes? The stress and strains transform
- In the constitutive relationship we have,

$$\underline{\underline{\sigma}}_{(1,2)} = \underline{\underline{C}} \underline{\underline{\varepsilon}}_{(1,2)}$$

$$\underline{\underline{T}} \underline{\underline{\sigma}}_{(1,2)} = \underline{\underline{\sigma}}_{(x,y)} = \underbrace{\underline{\underline{T}} \underline{\underline{C}} \underline{\underline{T}}^{-1}}_{\underline{\underline{C}}'} \underline{\underline{\varepsilon}}_{(x,y)}$$

where

$$\underline{\underline{C}} = \begin{bmatrix} C_{11} & C_{12} & 0\\ C_{12} & C_{22} & 0\\ 0 & 0 & C_{33} \end{bmatrix}$$

Analysis of Planar Laminates

Analysis of Planar Laminates

$$\begin{bmatrix} u_{x} \\ u_{y} \end{bmatrix} = \underbrace{\begin{bmatrix} \cos \theta & -\sin \theta \\ \cos \theta \end{bmatrix}}_{\begin{bmatrix} u_{2} \end{bmatrix}}^{\begin{bmatrix} u_{1} \\ u_{2} \end{bmatrix}}$$

$$\stackrel{(u_{2})}{\underline{\subseteq}}$$

$$\boxed{\underline{\subseteq}} \quad \underbrace{[u_{x}, y] = \underline{\subseteq} \underline{\subseteq} (1, 2) \underline{\subseteq}^{T}}_{\underline{\subseteq} (1, 2) \underline{\subseteq}^{T}} \quad \bullet \text{ What if the coordinate system is aligned with the fiber axes?}$$

$$\operatorname{stress and strains transform}_{aligned with the fiber axes?} \quad \operatorname{stress and strains transform}_{aligned with the fiber axes?}$$

$$\underline{\subseteq} ' = \begin{bmatrix} C_{11}' & C_{12}' & C_{13}' \\ C_{12}' & C_{22}' & C_{23}' \\ C_{13}' & C_{23}' & C_{33}' \end{bmatrix}_{a_{1}}^{a_{1}} \quad C_{11} + C_{22} + (2C_{33} + C_{12})2c^{2}s^{2} \\ C_{22}' = C_{11}s^{4} + C_{22}c^{4} + (2C_{33} + C_{12})2c^{2}s^{2} \\ C_{13}' = (C_{11} + C_{22} - 2C_{33} - 2C_{12})c^{2}s^{2} + C_{33}(c^{4} + s^{4}) \\ C_{12}' = (C_{11} + C_{22} - 4C_{33})c^{2}s^{2} + C_{12}(c^{4} + s^{4}) \\ C_{13}' = (-C_{11} + 2C_{33} + C_{12})c^{3}s + (C_{22} - 2C_{33} - C_{12})cs^{3} \\ C_{23}' = (C_{22} - 2C_{33} - C_{12})c^{3}s - (C_{11} - 2C_{33} - C_{12})cs^{3} \\ \vdots \\ \underline{T} = \begin{bmatrix} \cos^{2} \theta & \sin^{2} \theta & 2\cos \theta \sin \theta \\ \sin^{2} \theta & \cos^{2} \theta & -2\cos \theta \sin \theta \\ -\cos \theta \sin \theta & \cos \theta \sin \theta & \cos^{2} \theta - \sin^{2} \theta \end{bmatrix}$$

$$e^{\sum \left[\sum_{i=1}^{C_{11}} C_{12} & 0 \\ C_{12} & C_{22} & 0 \\ 0 & 0 & C_{33} \end{bmatrix}}.$$

Analysis of Planar Laminates

• Based on this we can write,

$$\begin{aligned} \text{Compliance is often more convenient:} \\ E_x &= \left[\frac{c^4}{E_1} + \frac{s^4}{E_2} + \left(\frac{1}{G_{12}} - \frac{2\nu_{21}}{E_2}\right)c^2s^2\right]^{-1} \\ &= \left[\frac{s^2}{Y_{12}}\right] = \left[\frac{s'_{11}}{S'_{22}} + \frac{s'_{13}}{S'_{33}}\right] \left[\frac{\sigma_x}{\tau_y}\right] \\ &= \left[\frac{s'_{11}}{S'_{12}} + \frac{s'_{12}}{S'_{23}} + \frac{s'_{13}}{S'_{33}}\right] \left[\frac{\sigma_x}{\tau_y}\right] \\ &= \left[\frac{s'_{11}}{S'_{12}} + \frac{s'_{12}}{S'_{22}} + \frac{s'_{13}}{S'_{33}}\right] \left[\frac{\sigma_x}{\tau_y}\right] \\ &= \left[\frac{s'_{11}}{S'_{12}} + \frac{s'_{22}s^4}{S'_{33}} + \frac{(1-2)s^2s^2}{S'_{33}}\right] \\ &= \left[\frac{s'_{11}}{S'_{12}} + \frac{s'_{22}s^4}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}}\right] \\ &= \left[\frac{s'_{11}}{S'_{12}} + \frac{s'_{22}s^4}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}}\right] \\ &= \left[\frac{s'_{11}}{S'_{12}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}}\right] \\ &= \left[\frac{s'_{11}}{S'_{12}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}}\right] \\ &= \left[\frac{s'_{11}}{S'_{12}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}}\right] \\ &= \left[\frac{s'_{11}}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}}\right] \\ &= \left[\frac{s'_{11}}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}}\right] \\ &= \left[\frac{s'_{11}}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}}\right] \\ &= \left[\frac{s'_{11}}{S'_{23}} + \frac{(1-2)s^2s^2}{S'_{23}} + \frac{(1-2)s^2$$

• In the material principal directions we have, 201

$$\begin{bmatrix} \varepsilon_1\\ \varepsilon_2\\ \gamma_{12} \end{bmatrix} = \begin{bmatrix} \frac{E_1}{E_1} & -\frac{E_1}{B_2} & 0\\ -\frac{V_1}{E_1} & E_2 & 0\\ 0 & 0 & \frac{1}{G_{12}} \end{bmatrix} \begin{bmatrix} \sigma_1\\ \sigma_2\\ \tau_{12} \end{bmatrix}$$

Engineering Constants: $E_1, E_2, G_{12}, \nu_{12}$

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• It is customary to express the laminate constitutive relationship as

$$\begin{bmatrix} \varepsilon_x \\ \varepsilon_y \\ \varepsilon_y \\ \gamma_{xy} \end{bmatrix} = \begin{bmatrix} \frac{1}{E_x} & -\frac{\nu_{yx}}{E_y} & \frac{\eta_{xy,x}}{\sigma_{xy}} \\ -\frac{\nu_{xy}}{E_x} & \frac{1}{E_y} & \frac{\eta_{xy,y}}{\sigma_{xy}} \\ \frac{\eta_{x,xy}}{E_x} & \frac{\eta_{y,xy}}{E_y} & \frac{1}{\sigma_{xy}} \end{bmatrix} \begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \\ \tau_{xy} \end{bmatrix}$$



5.2. Numerical Examples: 1

Analysis of Planar Laminates(Example 2.2 from Gibson 2012)

Consider an orthotropic laminate with the properties

 $E_1 = 140 \text{ GPa}, E_2 = 10 \text{ GPa}, G_{12} = 7 \text{ GPa}, \nu_{12} = 0.3, \nu_{23} = 0.2.$

Compute the strains if it is subjected to the following state of stress in the principal coordinates:

 $\sigma_1 = 70 \text{ MPa}, \sigma_2 = 140 \text{ MPa}, \tau_{12} = 35 \text{ MPa}, \sigma_3 = \tau_{12} = \tau_{23} = 0.$



(Figure 2.10 from Gibson 2012)

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5.2. Numerical Examples: 2

Analysis of Planar Laminates(Example 2.3 from Gibson 2012)

A 45° off-axis tensile test is conducted on a generally orthotropic test specimen by applying a normal stress σ_x . The specimen has strain gauges attached to measure axial and transverse strains ($\varepsilon_x, \varepsilon_y$). How many engineering parameters can be estimated from measurements of $\sigma_x, \varepsilon_x, \varepsilon_y$?



(Figure 2.15 from Gibson 2012)

6. Classical Laminate Theory

• In the Kirchhoff-Love Plate Theory we had,

$$\begin{bmatrix} \underline{N} \\ \underline{M} \end{bmatrix} = \begin{bmatrix} \underline{\underline{A}} & \underline{\underline{B}} \\ \underline{\underline{B}} & \underline{\underline{D}} \end{bmatrix} \begin{bmatrix} \underline{\underline{u}'} \\ \underline{\underline{w}''} \end{bmatrix}$$

where

$$\underline{\underline{A}} = \frac{Et}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0\\ \nu & 1 & 0\\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix}, \quad \underline{\underline{D}} = \frac{Et^3}{12(1-\nu^2)} \begin{bmatrix} 1 & \nu & 0\\ \nu & 1 & 0\\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix}, \quad \underline{\underline{B}} = \underline{\underline{0}}.$$

• This can also be written in terms of thickness moments of the constitutive matrix $\underline{\underline{C}} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix}$ as $\underline{\underline{A}} = \int_{-\frac{t}{2}}^{\frac{t}{2}} \underline{\underline{C}} dz, \quad \underline{\underline{B}} = \int_{-\frac{t}{2}}^{\frac{t}{2}} z \underline{\underline{C}} dz, \quad \underline{\underline{D}} = \int_{-\frac{t}{2}}^{\frac{t}{2}} z^2 \underline{\underline{C}} dz.$

6. Classical Laminate Theory

- Suppose we had different laminate plies along the thickness, such that the constitutive matrix is $\underline{\underline{C}}_i$ for $z \in (z_i, z_{i+1})$ and $-\frac{t}{2} = z_1 < \cdots < z_N = \frac{t}{2}$.
- Then the A B D matrices are written as the sums,

$$\underline{\underline{A}} = \sum_{i} (z_{i+1} - z_i) \underline{\underline{C}}_i, \quad \underline{\underline{B}} = \sum_{i} \frac{z_{i+1}^2 - z_i^2}{2} \underline{\underline{C}}_i, \quad \underline{\underline{D}} = \sum_{i} \frac{z_{i+1}^3 - z_i^3}{3} \underline{\underline{C}}_i.$$

- Unlike isotropic plates, composite laminates can have non-zero $\underline{\underline{B}}$ matrix (moment-planar coupling), bending-twisting coupling, etc.
- This $\begin{bmatrix} \underline{\underline{A}} & \underline{\underline{B}} \\ \underline{\underline{\underline{B}}} & \underline{\underline{\underline{D}}} \end{bmatrix}$ matrix is known as the **Laminate Compliance Matrix**.

6.1. The Laminate Orientation Code

Classical Laminate Theory

- Ply angles separated by slashes, ordered from top to bottom
- Subscript "s" for symmetric laminates
- Numerical subscripts for repetitions
- Center ply with an overbar for odd laminates

(See sec. 7.1 in Gibson 2012)

Types

- Symmetric, Antisymmetric, Asymmetric
- Angle-Ply, Cross-Ply, Balanced, $\pi/4$ laminates













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6.1. The Laminate Orientation Code Summary of Laminate Stiffnesses

Classical Laminate Theory

Table 3.4. The [A], [B], [D] matrices for laminates. When the laminate is summatrical the [B] matrix is zero. Cross plu laminates are orthotronic

- Ply angles separ ordered from to
- Subscript "s" fo laminates
- Numerical subs repetitions
- Center ply with odd laminates

(See Typ

• Symmetric, Antis Asymmetric

• Angle-Ply, Crosslaminates

[A]			[B]		[D]		
Symm	etrica	ıl					
$\int A_{11}$	A_{12}	A16	0 0	2	$\int D_{11}$	D_{12}	D_{16}
A_{12}	A22	A26	0 0 0	2	D12	D_{22}	D_{26}
A_{16}	A_{26}	A_{66}	0 0 0	0	D_{16}	D_{26}	D_{66}
Balan	ced						
[An	A12	0]	$\begin{bmatrix} B_{11} & B \end{bmatrix}$	12 B16	$\int D_{11}$	D_{12}	D16
A12	A22	0	B12 B	22 B 26	D12	D22	D26
0	0	A 66	B_{16} B	26 B ₁₆	D_{16}	D ₂₆	D 66
Ortho	tropic						
$\int A_{11}$	A12	0]	B ₁₁ B	12 0	$\int D_{11}$	D_{12}	0]
A12	A22	0	B12 B	22 0	D12	D_{22}	0
0	0	A_{66}	0 0	B ₆₆	0	0	D ₆₆
Isotro	pic						
$\int A_{11}$	A_{12}	0	$\begin{bmatrix} B_{11} & B \end{bmatrix}$	12 0	$\int D_{11}$	D_{12}	0
A12	A_{11}	0	B12 B	11 0	D12	D_{11}	0
0	0	$\frac{A_{11}-A_{12}}{2}$		$\frac{B_{11}-B_{12}}{2}$		0	$\frac{D_{11}-D_{12}}{2}$
Quasi	-isotro	opic					
$\int A_{11}$	A12	0	$\begin{bmatrix} B_{11} & B \end{bmatrix}$	12 B16	$\int D_{11}$	D_{12}	D_{16}
A12	A_{11}	0	B12 B	22 B26	D12	D22	D26
0	0	$\frac{A_{11}-A_{12}}{2}$	B_{16} B	26 B ₆₆	D_{16}	D_{26}	D 66



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6.2. Laminated Beams

Classical Laminate Theory

- Consider a beam on the x y plane. Invoking Kirchhoff kinematic assumptions we have: $\varepsilon_x = u' yv''$.
- The stress distribution will depend on the section-coordinate. In general we will have: $\sigma_x = E_x(y)\varepsilon_x = E_x(y)(u' yv'')$.
- We get the effective normal reaction N_x by integrating the stress over the section:

$$N_x = \int_{\mathcal{A}} \sigma_x = \left[\int_{\mathcal{A}} E_x(y) \right] u' + \left[\int_{\mathcal{A}} -y E_x(y) \right] v''.$$

• Similarly we get the bending moment M_z as the first moment of the stress,

$$M_z = \int_{\mathcal{A}} -y\sigma_x = \left[\int_{\mathcal{A}} -yE_x(y)\right]u' + \left[\int_{\mathcal{A}} y^2 E_x(y)\right]v''.$$

• In summary we have,

$$\begin{bmatrix} N_x \\ M_z \end{bmatrix} = \begin{bmatrix} A & B \\ B & D \end{bmatrix} \begin{bmatrix} u' \\ v'' \end{bmatrix}.$$

6.2. Laminated Beams

Classical Laminate Theory

• For a laminated composite, the integrals may be simplified as,

$$A = \int_{\mathcal{A}} E_x(y) = \sum_{i=1}^{N} E_{x,i}(y_{i+1} - y_i), \quad B = \int_{\mathcal{A}} -y E_x(y) = \sum_{i=1}^{N} E_{x,i} \frac{y_{i+1}^2 - y_i^2}{2}$$
$$D = \int_{\mathcal{A}} y^2 E_x(y) = \sum_{i=1}^{N} E_{x,i} \frac{y_{i+1}^3 - y_i^3}{3}.$$

• For plies of uniform thickness we can write

$$y_j = -\frac{h}{2} + (j-1)\frac{h}{N-1},$$

which leads to:

$$A = \frac{h}{N} \sum_{i=1}^{N} E_{x,i}, B = \frac{h^2}{2N^2} \sum_{i=1}^{N} E_{x,i}(2i - N - 1), D = \frac{h^3}{12N^3} \sum_{i=1}^{N} E_{x,i}(\dots)$$

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